

1. BUILDING CODE - INTERNATIONAL BUILDING CODE 2018 WITH 2020 GEORGIA AMENDMENTS		
2. RISK CATEGORY & IMPORTANCE FACTORS:		
A. RISK CATEGORY.....	II	
B. WIND FACTOR.....	1.0	
C. SNOW FACTOR.....	1.0	
D. SEISMIC FACTOR.....	1.0	
3. DESIGN DEAD LOADS:		
A. ROOF.....	20 PSF	
B. TOP CHORD DEAD LOAD.....	10 PSF	
C. BOTTOM CHORD DEAD LOAD.....	10 PSF	
4. DESIGN LIVE LOADS*:		
A. SLAB ON GRADE.....		
LOBBY/CORRIDOR.....	100 PSF	
OFFICES.....	50 PSF	
B. ROOF TOP CHORD LIVE LOAD.....	20 PSF	
4. WIND LOADS:		
A. ULTIMATE WIND SPEED.....	106 MPH	
B. DIRECTIONALITY FACTOR (Kd).....	0.85	
C. EXPOSURE CATEGORY.....	B	
D. ENCLOSURE CLASSIFICATION.....	ENCLOSED BUILDING	
E. GUST EFFECT FACTOR (G).....	0.85	
5.		
6. SNOW LOADS:		
A. GROUND SNOW LOAD.....	5.0 PSF	
B. THERMAL FACTOR.....	1.0	
C. EXPOSURE FACTOR.....	1.0	
D. RAIN ON SNOW SURCHARGE.....	5.0 PSF	
E. UNIFORM ROOF SNOW LOAD.....	8.5 PSF	

- STRUCTURAL DRAWINGS SHALL BE COORDINATED WITH ARCHITECTURAL AND MECHANICAL DRAWINGS. CONTRACTOR IS RESPONSIBLE FOR COORDINATING PERTINENT ASPECTS OF ALL DISCREPANCIES INTO THEIR SHOP DRAWINGS AND WORK, AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES OR OMISSIONS.
2. NO OPENINGS OR MODIFICATIONS SHALL BE MADE IN OR TO ANY STRUCTURAL MEMBER WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
3. NO CHANGE IN SIZE OR DIMENSION OF STRUCTURAL MEMBERS SHALL BE MADE WITHOUT THE WRITTEN APPROVAL OF THE ARCHITECT.
4. OPENINGS 1'-4" OR LESS ON A SIDE ARE GENERALLY NOT SHOWN ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR SUCH OPENINGS.
5. THE CONTRACTOR IS RESPONSIBLE FOR LIMITING THE AMOUNT OF CONSTRUCTION LOAD IMPOSED UPON STRUCTURAL FRAMING. CONSTRUCTION LOADS SHALL NOT EXCEED THE DESIGN CAPACITY OF THE FRAMING AT THE TIME THE LOADS ARE IMPOSED.
6. THE STRUCTURE IS DESIGNED TO FUNCTION AS A UNIT UPON COMPLETION. THE CONTRACTOR IS RESPONSIBLE FOR FURNISHING ALL THE TEMPORARY BRACING AND/OR SUPPORT THAT MAY BE REQUIRED AS THE RESULT OF THE CONTRACTOR'S CONSTRUCTION METHODS AND/OR SEQUENCES.
7. DO NOT SCALE THESE DRAWINGS: USE DIMENSIONS. FOR DIMENSIONS NOT SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS, SEE ARCHITECTURAL DRAWINGS.
8. CONTRACTOR'S CONSTRUCTION AND/OR ERECTION SEQUENCES SHALL RECOGNIZE AND CONSIDER THE EFFECTS OF THERMAL MOVEMENTS OF STRUCTURAL ELEMENTS DURING THE CONSTRUCTION PERIOD.
9. THE CONTRACTOR SHALL INFORM THE PROFESSIONAL OF RECORD, IN WRITING, OF ANY DEVIATION FROM THE CONTRACT DOCUMENTS. THE CONTRACTOR SHALL NOT BE RELIEVED OF THE RESPONSIBILITY OF SUCH DEVIATION BY THE PROFESSIONAL OF RECORD, REVIEW OF SHOP DRAWINGS, PRODUCT DATA, ETC. UNLESS THE CONTRACTOR HAS SPECIFICALLY INFORMED THE PROFESSIONAL OF RECORD OF SUCH DEVIATION AT THE TIME OF SUBMISSION AND THE ARCHITECT HAS GIVEN THE WRITTEN APPROVAL TO THE SPECIFIC DEVIATION.
10. WHERE A SECTION/DETAIL IS CUT ON THE PLAN, IT IS ASSUMED/UNDERSTOOD TO BE REPRESENTATIVE OF ALL LIKE OR SIMILAR CONDITIONS. THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING SUCH REQUIREMENTS INTO THEIR SHOP DRAWINGS AND WORK.
11. AT ALL TIMES THE CONTRACTOR SHALL BE SOLELY RESPONSIBLE FOR THE CONDITIONS OF THE JOB SITE, INCLUDING SAFETY OF PERSONS AND PROPERTY. THE ARCHITECT'S OR ENGINEER'S PRESENCE AT THE JOB SITE OR REVIEW OF WORK DOES NOT IMPLY CONFIRMATION OF THE ADEQUACY OF THE CONTRACTOR'S MEANS OR METHODS OF CONSTRUCTION. THE CONTRACTOR IS RESPONSIBLE FOR COMPLIANCE WITH OSHA REGULATIONS.
12. CONSULT ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS FOR LOCATION, SIZE AND EXTENT OF CHASES, INSERTS, RECESSES, RIDGES, FINISHES, DEPRESSIONS, ETC., NOT SHOWN ON THE STRUCTURAL DRAWINGS.
13. THE GENERAL CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND SITE CONDITIONS BEFORE STARTING WORK. THE ARCHITECT SHALL BE NOTIFIED OF ANY DISCREPANCIES.
14. THE CONTRACTOR SHALL VERIFY ALL ROOF MOUNTED MECHANICAL EQUIPMENT WEIGHTS AS WELL AS ROOF OPENING SIZES AND LOCATIONS WITH ARCHITECTURAL AND MECHANICAL DRAWINGS.
15. THE CONTRACTOR SHALL NOTIFY, IN WRITING, THE STRUCTURAL ENGINEER OF RECORD OF CONDITIONS ENCOUNTERED IN THE FIELD WHICH ARE CONTRADICTORY TO THOSE SHOWN ON THE STRUCTURAL CONTRACT DOCUMENTS.
16. STRUCTURAL CONTRACT DOCUMENTS SHALL NOT INCLUDE SHOP DRAWINGS, VENDOR DRAWINGS, OR ANY MATERIAL PREPARED AND SUBMITTED BY THE CONTRACTOR OR SUBCONTRACTOR.
17. SEE ARCHITECTURAL DRAWINGS FOR FLOOR ELEVATIONS, SLOPE, AND LOCATION OF DEPRESSED FLOOR AREAS. THE CONTRACTOR SHALL COMPARE STRUCTURAL SECTIONS WITH THE ARCHITECTURAL SECTIONS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATING OR INSTALLING STRUCTURAL MEMBERS.
18. PRINCIPAL OPENINGS THROUGH THE FRAMING ARE SHOWN ON THESE DRAWINGS. THE GENERAL CONTRACTOR SHALL EXAMINE THE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR THE REQUIRED OPENINGS AND HE SHALL PROVIDE FOR ALL OPENINGS WHETHER SHOWN ON THE DRAWINGS OR NOT. HE SHALL VERIFY SIZE AND LOCATION OF ALL OPENINGS WITH THE MECHANICAL CONTRACTOR, ANY DEVIATION FROM THE OPENINGS SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE BROUGHT TO THE ENGINEER'S ATTENTION FOR APPROVAL.

NOTE: AMERICAN CONCRETE INSTITUTE (ACI) 318 (LATEST ADDITION)

2. CONCRETE SHALL HAVE A 28-DAY COMPRESSIVE STRENGTH AND DENSITY IN ACCORDANCE WITH THE FOLLOWING:

	STRENGTH	DENSITY
	PSI	PCF
FOOTINGS & SLABS ON GRADE.....	3000	145

3. CONTRACTOR SHALL SUBMIT CONCRETE MIX DESIGNS FOR REVIEW WELL IN ADVANCE OF CONCRETE PLACEMENT. CONCRETE MIX DESIGN SHALL INCLUDE ALL STRENGTH DATA NECESSARY TO SHOW COMPLIANCE WITH THE PROJECT SPECIFICATIONS BY EITHER THE TRIAL BATCH OR FIELD EXPERIENCE METHOD, AND SHALL BE CERTIFIED BY AN ENGINEER REGISTERED IN THE PROJECT STATE.

4. REINFORCING SHALL CONFORM TO ASTM A615, GR60, UNLESS NOTED OTHERWISE.

5. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 60.

6. WELDED WIRE FABRIC SHALL BE PLACED 1" BELOW T/SLAB, UNLESS NOTED OTHERWISE. LAP FABRIC 6" ON SIDES AND ENDS.

7. ALL REINFORCING SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH THE LATEST ADDITION OF THE ACI DETAILING MANUAL.

8. ALL MIXING, TRANSPORTING, PLACING AND CURING OF CONCRETE SHALL BE DONE IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN CONCRETE INSTITUTE.

9. THERE SHALL BE NO HORIZONTAL CONSTRUCTION JOINTS IN ABOVE-GRADE CONCRETE POURS. ALL CONSTRUCTION JOINTS SHALL BE MADE IN THE CENTER OF SPANS WITH VERTICAL BULKHEADS. WHEN A BEAM INTERSECTS A GIRDER AT THIS POINT, THE JOINTS OF THE GIRDERS SHALL BE OFFSET A DISTANCE EQUAL TO TWICE THE WIDTH OF THE BEAM. THE LOCATION OF CONSTRUCTION JOINTS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD. ADDITIONAL REINFORCING AT CONSTRUCTION JOINTS SHALL BE SPECIFIED BY THE ENGINEER OF RECORD.

10. ALL "CONTINUOUS" REINFORCEMENT SHALL HAVE A MINIMUM LAP OF "B" TYPE (ACI 318) AT SPLICES, UNLESS NOTED OTHERWISE.

11. PROVIDE STANDARD BAR CHAIRS WITH PROTECTIVE TIPS AND SPACERS AT 5'-0" CENTERS FOR ALL SLABS AND BEAMS ABOVE GRADE. PROVIDE 3" X 6" X 20 GAGE SHEET METAL BAR CHAIRS AT 4'-0" MAXIMUM CENTERS EACH WAY FOR ALL TOP REINFORCING FOR SLABS-ON-GRADE.

12. SUBMIT REINFORCING PLACEMENT AND DETAIL (SHOP) DRAWINGS FOR REVIEW. NO REINFORCING BARS SHALL BE INSTALLED UNTIL THE SHOP DRAWINGS HAVE BEEN REVIEWED AND RETURNED.

13. PRODUCTS AND MATERIALS:

- A. TYPE 1 PORTLAND CEMENT SHALL CONFORM TO ASTM-C150.
- B. AGGREGATES SHALL CONFORM TO ASTM C-33.
- C. REINFORCING BARS SHALL CONFORM TO ASTM A-615 (GRADE 60).
- D. FORMING SHALL BE OF WOOD, STEEL, OR FIBERGLASS OF SATISFACTORY QUALITY AND CONDITION.
- E. NO ADMIXTURES SHALL BE ADDED TO THE CONCRETE UNLESS APPROVED BY THE ENGINEER.
- F. NON-SHRINK GROUT SHALL BE READY TO USE NON-METALLIC AGGREGATE AND DEVELOP A 7-DAY COMPRESSIVE STRENGTH OF 5000 PSI.

14. ALL REINFORCING SHALL BE SUPPORTED IN FORMS SPACED WITH NECESSARY ACCESSORIES AND SHALL BE SECURELY WIRED TOGETHER IN ACCORDANCE WITH LATEST ADDITION OF THE CRSI "MANUAL OF STANDARD PRACTICE".

15. MINIMUM CONCRETE COVER (UNLESS NOTED OTHERWISE) SHALL BE:

A. #11 BARS AND SMALLER.....	3/4 INCHES
B. UNFORMED SURFACE IN CONTACT WITH THE GROUND.....	3 INCHES
C. FORMED SURFACES EXPOSED TO EARTH OR WEATHER	
#6 BARS AND LARGER.....	2 INCHES
#5 BARS AND SMALLER.....	1 1/2 INCHES

16. LAP SPLICES SHALL BE IN ACCORDANCE WITH THE FOLLOWING, UNLESS NOTED OTHERWISE, WHERE CLASSES ARE NOT CALLED OUT ON DRAWINGS, USE CLASS "B" SPLICES.

BAR SIZE	TENSION SPLICES (INCHES)		COMPRESSION SPLICES (INCHES)	
	TOP BARS	OTHER BARS	A	B
3	22	17	22	12
4	29	27	22	15
5	36	47	28	19

17. SCHEDULED OR DETAILED REINFORCING STEEL SHALL NOT BE TACK WELDED FOR ANY REASON. WELDED REINFORCING STEEL SPLICES ARE NOT PERMITTED WITHOUT ENGINEER'S APPROVAL. WHERE WELDING IS APPROVED IT SHALL CONFORM TO AWS D1.4 STRUCTURAL WELDING CODE - REINFORCING STEEL.

18. CORNER BARS SHALL BE OF EQUAL SIZE AND SPACING AS THE MAIN REINFORCING WITH LAP SPLICED LENGTHS EQUAL TO 44 BAR DIAMETERS, MINIMUM.

19. SLAB-ON-GRADE SHALL BE SAW CUT IMMEDIATELY AFTER CONCRETE HARDENS. THE CONTRACTOR SHALL SUBMIT LAYOUT AND CONSTRUCTION SCHEDULE ("SOFT CUT" ® INTERNATIONAL OR SIM.)

20. CONTROL JOINTS IN SLABS ON GROUND SHALL BE LOCATED AT 15'-0" MAXIMUM SPACING AND SHALL CREATE SECTIONS OF SLAB WITH A MAXIMUM ASPECT RATIO OF 1.5:1. CONTROL JOINTS SHALL BE SAWN AND SHALL BE A MINIMUM OF 1/2 OF THE SLAB THICKNESS DEEP IF CUT WITH A CONVENTIONAL SAW, OR 1" DEEP IF CUT WITH AN EARLY-ENTRY DRY-CUT SAW. THE CONTROL JOINTS SHALL BE SAWN AS SOON AS THE SAW BLADE CAN CUT THE CONCRETE WITHOUT DISPLACING THE AGGREGATE. CUT EVERY OTHER MESH WIRE AT THE CONTROL JOINT LOCATION PRIOR TO PLACING CONCRETE.

21. PULLING CONTROL JOINTS SHALL BE PLACED AS SOON AS CONCRETE IS ABLE TO BE SAWN WITHOUT SAWN AGGREGATE FROM FLOOR. SLABS SHALL NOT BE LEFT OVERNIGHT, OR ANY REASONABLE AMOUNT OF TIME, WITHOUT SAWING JOINTS. WEATHER IS CRITICAL TO THE SCHEDULE OF SAWN JOINTS. IF LARGE AREAS OF SLAB ARE POURED AT ONE TIME, SEVERAL SAWS MAY BE REQUIRED SO THAT JOINTS ARE PLACED IN TIME TO PREVENT SHRINKAGE CRACKING. PROPER JOINTING OF THE SLAB IS CRITICAL. REFER TO THE ACI MANUAL OF CONCRETE PRACTICE FOR PROPER JOINTING TECHNIQUES.

22. THE FLATNESS AND LEVELNESS OF THE SLAB-ON-GRADE SHALL BE DETERMINED ACCORDING TO ASTM E-1155 OR ACI 117. SLAB CLASS 5 (ACI 302) STANDARD TEST METHOD USING F NUMBERS. THE SPECIFIC FLATNESS AND LEVELNESS SHALL BE F/1-35 AND FL-20.

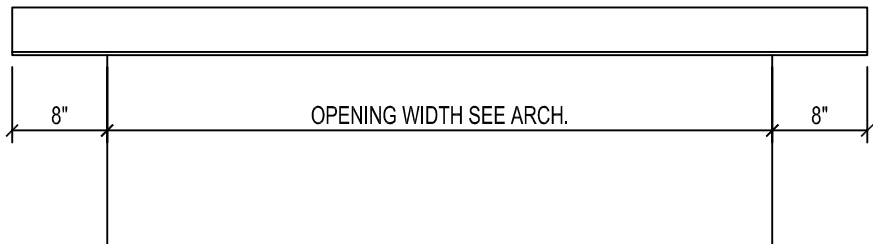
23. WHERE FOOTINGS, WALLS, OR OTHER STRUCTURAL ELEMENTS INTERSECT, CORNER OR TEE, PROVIDE CORNER BARS WITH REQUIRED LAP LENGTHS TO PROVIDE CONTINUITY OF HORIZONTAL STEEL REINFORCING. UNLESS NOTED OTHERWISE.

24. WHERE DOWELS, BOLTS OR INSERTS ARE CALLED OUT TO BE ANCHORED TO CAST IN PLACE OR PRECAST CONCRETE ELEMENTS USING ADHESIVE ANCHORS, USE AN ANCHORAGE SYSTEM EQUAL TO "HILTI" HIT HY-200. FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS. ALTERNATE ANCHORAGE SYSTEMS MAY BE USED WITH ENGINEER'S PRIOR APPROVAL.

25. PLACEMENT OF CONCRETE, COLD WEATHER AND HOT WEATHER PRECAUTIONS, MATERIAL AND PROPORTIONING REQUIREMENTS, REBAR COVER AND DETAILING SHALL CONFORM TO THE REQUIREMENTS OF THE ACI 318.

1. SPREAD FOOTINGS SHALL BE DESIGNED FOR A SOIL BEARING PRESSURE OF 2000 PSF.
2. THE FOOTINGS HAVE BEEN POSITIONED AT THE ESTIMATED ELEVATION WHICH WILL PROVIDE SUITABLE BEARING. HOWEVER, IF ADEQUATE BEARING CAPACITY IS NON-EXISTENT AT THESE ESTIMATED ELEVATIONS, THE FOOTING SHALL BE LOWERED TO AN ELEVATION WHERE THE PRESCRIBED SAFE BEARING CAPACITY EXISTS.
3. FOOTINGS MAY BE CAST INTO AN EARTH-FORMED TRENCH IF SOIL CONDITIONS PERMIT.
4. EXCAVATION FOR FOOTINGS SHALL BE CUT TO ACCURATE SIZES AND DIMENSIONS, AS SHOWN ON PLANS. ALL SOIL BELOW SLABS AND FOOTINGS SHALL BE PROPERLY COMPACTED AND SUBGRADE BROUGHT TO A REASONABLE TRUE AND LEVEL PLANE BEFORE PLACING CONCRETE.
5. IN THE AREA OF THE BUILDING, EXISTING ORGANIC MATERIAL, UNSUITABLE SOIL, ABANDONED FOOTINGS AND ANY OTHER EXISTING UNSUITABLE MATERIALS SHALL BE REMOVED. ANY FILL MATERIAL REQUIRED AT THE SITE SHALL BE OF A SIMILAR TYPE SOIL TO THAT WHICH IS PRESENT AT THIS SITE AND APPROVED BY A SOILS ENGINEER. ROCKS GREATER THAN 6 INCHES SHALL BE EXCLUDED FROM STRUCTURAL FILL LIFTS. FILL MATERIAL SHALL BE PLACED IN LOOSE LIFTS NO GREATER THAN 12 INCHES IN DEPTH AND SHALL BE COMPACTED TO AT LEAST 95% OF THE MATERIAL'S MAXIMUM DRY DENSITY AS DETERMINED BY THE MODIFIED COMPACTION TEST (ASTM D1557). ADEQUATE FIELD DENSITY AND MOISTURE CONTENT TESTS SHALL BE PERFORMED TO ENSURE COMPLIANCE.
6. FOOTING CONCRETE SHALL BE CAST ON THE SAME DAY THE EXCAVATION IS APPROVED. IF THE BEARING SURFACE IS ALLOWED TO BECOME DISTURBED IN ANY WAY, IT SHALL BE REWORKED TO THE SATISFACTION OF THE TESTING ENGINEER PRIOR TO CASTING THE CONCRETE.
7. ALL BEARING MATERIAL SHALL BE INSPECTED BY THE INDEPENDENT TESTING AGENCY PRIOR TO CONCRETE PLACEMENT. THE INDEPENDENT TESTING AGENCY SHALL BE THE SOLE JUDGE AS TO THE SUITABILITY OF THE BEARING MATERIAL. FOOTING ELEVATIONS SHALL BE ADJUSTED AS REQUIRED.
8. WHEN UNSATISFACTORY OR UNCONTROLLED FILL IS ENCOUNTERED, REMOVAL AND REPLACEMENT WILL BE PAID ON THE BASIS OF UNIT PRICES SET FORTH IN THE CONTRACT.
9. ANY FILL WITHIN 10'-0" OF THE BUILDING LIMIT SHALL BE COMPACTED TO 95% STANDARD PROCTOR DENSITY. THE UPPER 12'-0" OF FILL BENEATH STRUCTURAL AREAS SHOULD BE COMPACTED TO 98% OF THE MAXIMUM STANDARD PROCTOR DENSITY.
10. NO EXCAVATION SHALL BE CLOSER THAN AT A SLOPE OF 2:1 (2 HORIZONTAL TO 1 VERTICAL) TO A FOOTING. PROVIDE SHORING AND PROTECTION FOR EXCAVATION BANKS AS NECESSARY TO PRESERVE SAFETY AND PREVENT CAVING.
11. ALL BEARING STRATA SHALL BE ADEQUATELY DRAINED BEFORE FOUNDATION CONCRETE IS PLACED.
12. BACKFILL AROUND AND OVER FOUNDATION ELEMENTS SHALL BE OF SUITABLE MATERIAL, INSPECTED AND PRE-APPROVED BY THE TESTING ENGINEER.
13. THERE SHALL BE NO HORIZONTAL OR VERTICAL CONSTRUCTION JOINTS IN ANY FOOTING WITHOUT PRIOR WRITTEN APPROVAL FROM THE ENGINEER.
14. CONCRETE CAST ON SLOPING SURFACES SHALL BEGIN AT THE LOWEST ELEVATION AND CONTINUE MONOLITHICALLY TOWARD THE HIGHER ELEVATION UNTIL THE INTENDED POUR IS COMPLETED.

- CODES: STRUCTURAL WOOD IS TO BE DESIGNED, DETAILED, FABRICATED AND CONSTRUCTED IN ACCORDANCE WITH THE LATEST ADDITIONS OF:
  - A. "NATIONAL DESIGN SPECIFICATIONS FOR WOOD CONSTRUCTION" (ANSI/AWC NDS) BY AMERICAN WOOD COUNCIL.
  - B. PRODUCT STANDARD PS 20 "AMERICAN SOFTWOOD LUMBER STANDARD" BY ALSC.
  - C. PLYWOOD CONFORMING TO APA-THE ENGINEERED WOOD ASSOCIATION.
  - D. METAL PLATE-CONNECTED WOOD TRUSS DESIGN CONFORMING TO "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES" BY TRUSS PLATE INSTITUTE (TPI) AND TPI QUALITY CONTROL MANUAL.
2. ALL TIMBER SHALL BE 2" SOUTHERN YELLOW PINE (MOISTURE CONTENT 19% MAX.) OR EQUAL UNLESS NOTED OTHERWISE.
3. ALL WOOD TO WOOD CONNECTIONS SHALL EMPLOY PRE-MANUFACTURED METAL ANCHORS. TOE OR END NAILING OF WOOD SHALL NOT BE PERMITTED UNLESS NOTED OTHERWISE. METAL ANCHORS SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY OR EQUAL.
4. MINIMUM OF 2-2x4 OR 2-2x6 STUDS SHALL BE LOCATED UNDER ALL BUILD-UP WOOD MEMBERS U.N.O.
5. TRUSS MEMBERS AND CONNECTOR PLATES SHALL BE DESIGNED IN ACCORDANCE WITH TRUSS PLATE INSTITUTE SPECIFICATIONS FOR THE LOADING STATED BELOW. CONNECTOR PLATES WITHIN 1 INCH OF EDGE OR END OF MEMBER AT ANY JOINT SHALL NOT BE CONSIDERED IN DEVELOPING STRESS.
6. ERECTION BRACING SHALL BE INSTALLED AS NECESSARY TO HOLD THE TRUSSES TRUE AND PLUMB AND IN SAFE CONDITION UNTIL PERMANENT TRUSS BRACING AND BRIDGING CAN BE INSTALLED. ALL ERECTION AND PERMANENT BRACING SHALL BE INSTALLED AND ALL COMPONENTS PERMANENTLY FASTENED BEFORE THE APPLICATION OF ANY LOADS TO THE TRUSSES. ALL TEMPORARY BRACING LOCATIONS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD FOR REVIEW ON SHOP DRAWINGS SUBMITTALS. ALL PREFABRICATED WOOD TRUSSES ARE TO BE INSTALLED IN ACCORDANCE WITH BRACING WOOD TRUSSES COMMENTARY (BWT-76) OR HFT-80, AS PUBLISHED BY THE TRUSS PLATE INSTITUTE.
7. PRE-ENGINEERED METAL PLATE CONNECTED WOOD TRUSSES SHALL BE BRACED IN ACCORDANCE WITH THE LATEST ADDITION OF THE TRUSS PLATE INSTITUTE'S "BUILDING COMPONENT SAFETY INFORMATION BOOKLET" AND RELATED SUMMARY SHEETS.
8. DESIGN OF TIMBER TRUSSES SHALL BE PERFORMED BY A STRUCTURAL ENGINEER LICENSED IN THE PROJECT STATE. STAMPED SHOP DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT FOR APPROVAL PRIOR TO FABRICATION. SHOP DRAWINGS SHALL BE SEALED BY THE DESIGN ENGINEER.
9. ALL TRUSS TO TRUSS CONNECTIONS SHALL BE DESIGNED BY THE TRUSS DESIGN ENGINEER. TRUSS DESIGN ENGINEER SHALL SPECIFY ALL HARDWARE REQUIRED FOR THE CONNECTIONS.
10. WOOD EMBEDDED OR PLACED ON CONCRETE IN DIRECT CONTACT WITH EARTH SHALL BE PRESSURE TREATED INCLUDING BUT NOT LIMITED TO POSTS, COLUMN SLEEPERS, SILLS AND SOLE PLATES.
11. ALL PRE-ENGINEERED WOOD TRUSSES SHALL BE BRACED IN ACCORDANCE WITH TRUSS PLATE INSTITUTE'S "HANDLING, INSTALLING AND BRACING METAL PLATE CONNECTED WOOD TRUSSES, HIB-91"
12. ALL PRE-ENGINEERED WOOD TRUSS SHOP DRAWINGS SHALL BE AVAILABLE ON THE JOB SITE DURING THE TIMES OF INSPECTION AND SHALL BEAR CLEAR INDICATION OF THAT THEY HAVE BEEN REVIEWED AND APPROVED BY THE PROJECT STRUCTURAL ENGINEER-OF-RECORD.
13. ALL NAILS, BOLTS, SCREWS, AND LAG SCREWS SHALL BE HOT-DIP GALVANIZED OR STAINLESS STEEL. WOOD CONNECTOR HARDWARE SHALL BE HOT-DIP GALVANIZED, "Z-MAX" GALVANIZED OR TYPE 316 STAINLESS STEEL. ALL GALVANIZED FASTENERS SHALL BE USED WITH GALVANIZED HARDWARE AND STAINLESS STEEL FASTENERS SHALL BE USED WITH STAINLESS STEEL HARDWARE.



BRICK LINTEL SCHEDULE			
OPENING WIDTH		STEEL LINTELS	
MIN.	MAX.		
—	6'-0"	L 4 x 4 x 1/4	
6'-1"	9'-4"	L 5 x 3 1/2 x 3/8 LLV	
9'-4"	12'-0"	L 5 x 3 1/2 x 3/8 LLV	

\* 8" BEARING EACH END FOR STEEL

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[illegible]

COUNTY ELECTIONS OFFICE  
REMODEL  
175 JOHNSON AVENUE  
FAYETTEVILLE, 30214 GEORGIA  
PROJECT # 31142

**PROJECT**



FINAL DRAWING  
FOR REVIEW PURPOSES ONLY  
Release Date: May 25, 2021

SEAL

DRAWING TITLE

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# SO



FAYETTE COUNTY ELECTIONS OFFICE  
REMODEL  
175 JOHNSON AVENUE  
FAYETTEVILLE, 30214 GEORGIA  
PROJECT #3142



FINAL DRAWING  
FOR REVIEW PURPOSES ONLY  
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SEAL

## FOUNDATION PLAN

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Drawn By:

SKE

PL

21-314

S1

SCALE: 1/4" = 1'-0"

PLAN NOTES

**PLAN NOTES:**

1.  $(X'-X'')$  INDICATES TOP OF FOOTING ELEVATION BELOW FINISHED FLOOR ELEVATION (F.F.E.) = 0'-0"
2. F# INDICATES A FOOTING MARK (SEE SCHEDULE).
3. FOR DIMENSIONS NOT SHOWN ON THE PLANS SEE ARCHITECT.


FOOTING SCHEDULE		
TAG	SIZE	REINFORCEMENT
F2	2'-0"x2'-0"x16"	(4) #4 E.W. BOTTOM
F3	3'-0"x3'-0"x16"	(6) #4 E.W. BOTTOM

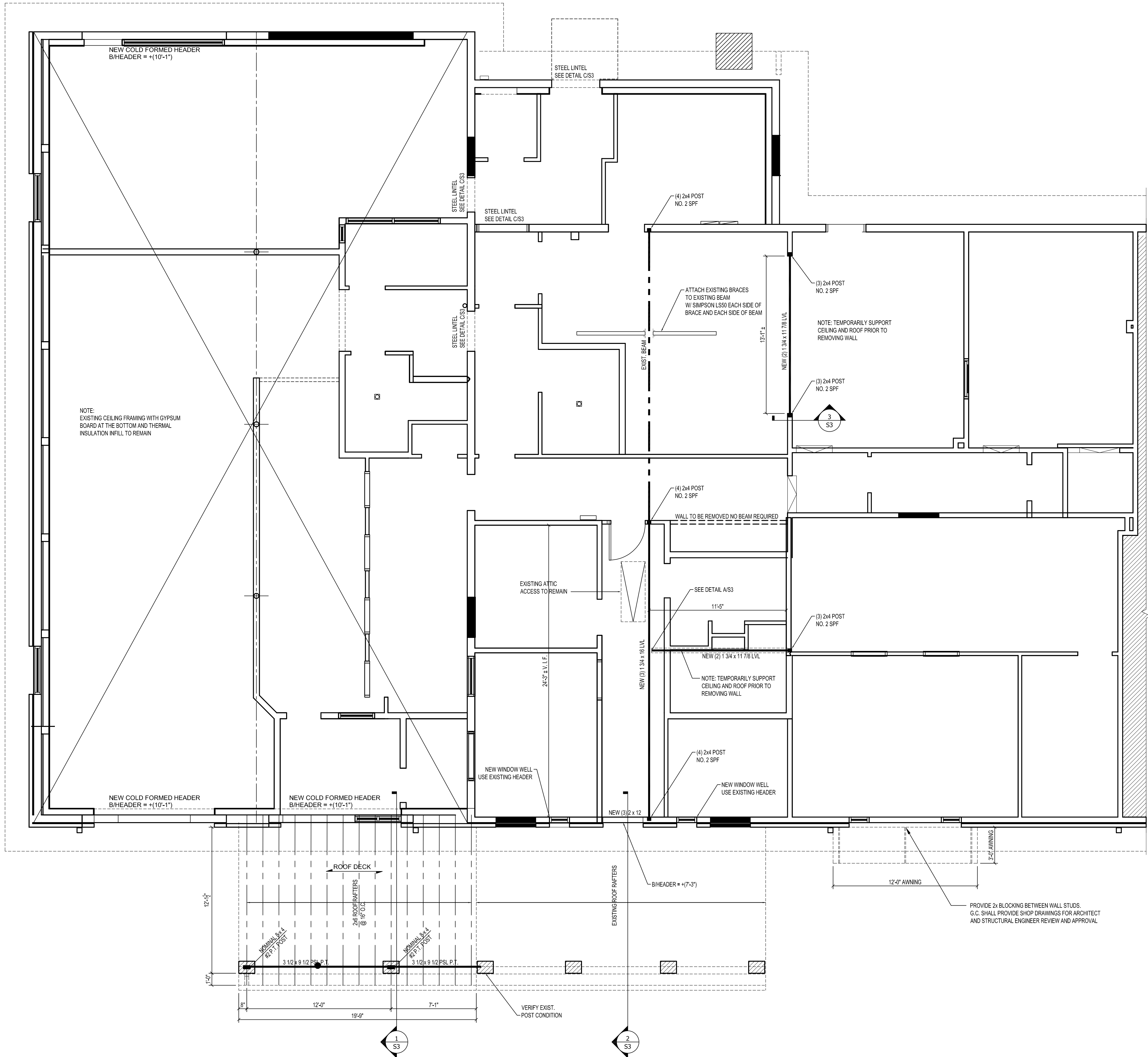


A  
S2

FRAMING PLAN

SCALE: 1/4" = 1'-0"

1.  ROOF DECK INDICATES SPAN OF 5/8" ROOF DECK ATTACHED WITH 8d NAILS SPACED AT 6" EDGES AND 6" INTERMEDIATE.
2. FOR DIMENSIONS NOT SHOWN ON THE PLANS SEE ARCHITECT.

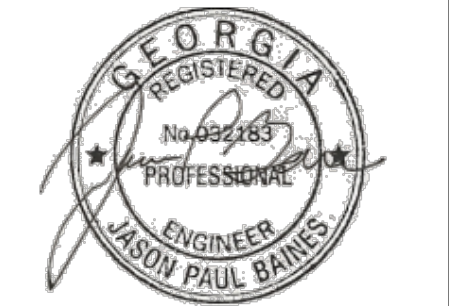


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PRINT RECORD  
# DESCRIPTION (DOT INDICATES SHEET WAS REVIEWED)

FAYETTE COUNTY ELECTIONS OFFICE  
REMODEL  
175 JOHNSON AVENUE  
FAYETTEVILLE, 30214 GEORGIA  
PROJECT #3142



FINAL DRAWING  
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FRAMING PLAN

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S2



